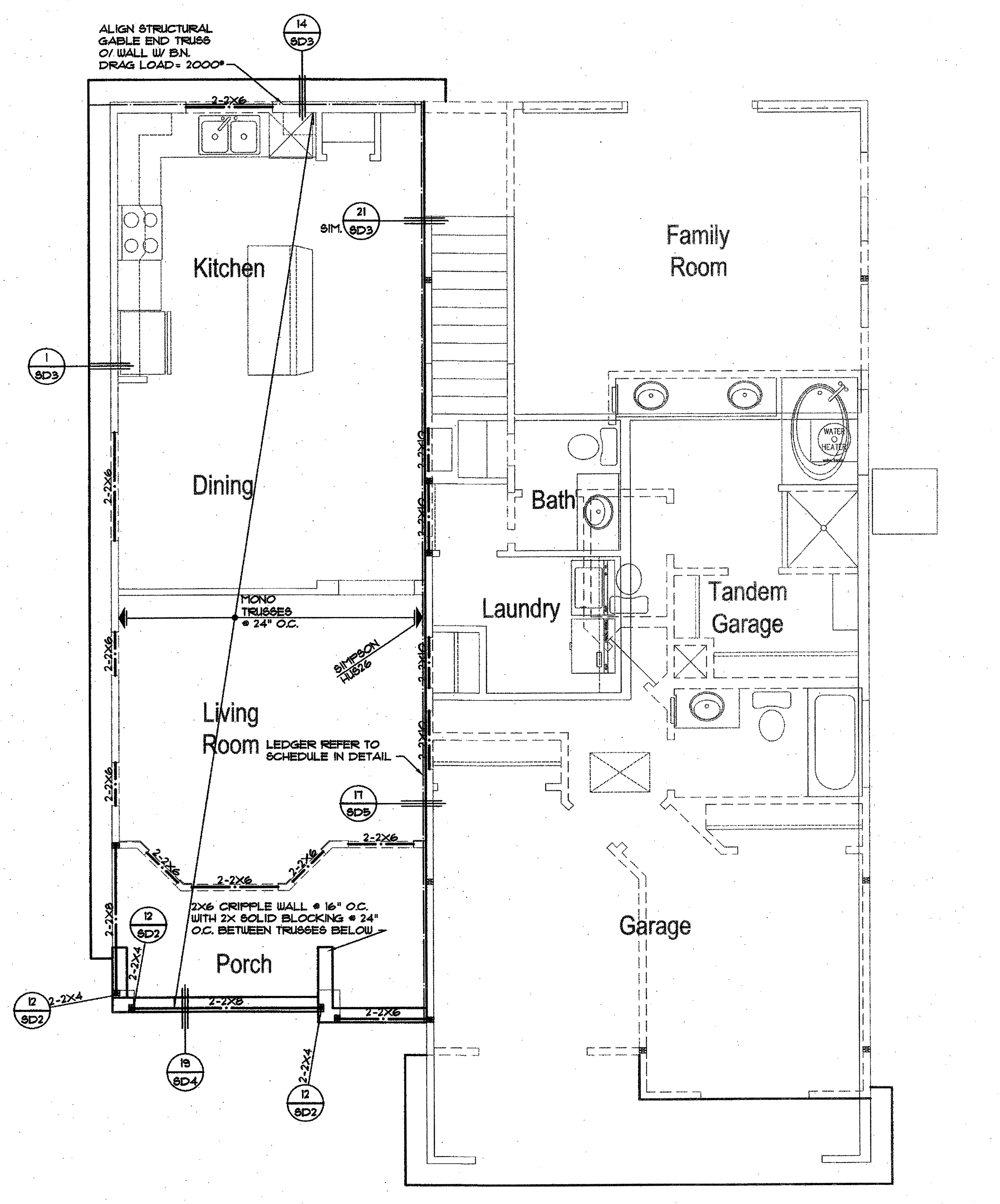


FLOOR FRAMING / LOWER SHEAR WALL PLAN C

REFER TO FLOOR FRAMING / LOWER SHEAR WALL PLAN 'A' FOR ALL INFORMATION NOT SHOWN. ONLY INFORMATION REVISED FROM FLOOR FRAMING / LOWER SHEAR WALL PLAN 'A' IS SHOWN FOR CLARITY.

GENERAL FRAMING NOTES, CONVENTIONAL, WOOD:

BAI NO.: GENWD-AZ

1. See specifications and notes on SN sheets for additional information and sheet SD2.0 for typical construction details.

STUD MATERIAL

Provide wall stud as shown below U.N.O. on plans or details.

2.

Location	Studs
Exterior bearing wall	2X4 Spruce-Pine-Fir #1 / #2 grade studs at 16" o.c.
Exterior nonbearing wall	2X4 Spruce-Pine-Fir #1 / #2 grade studs at 16" o.c.
Interior bearing wall	2X4 Spruce-Pine-Fir #1 / #2 grade studs at 16" o.c.
Interior nonbearing wall	2X4 Spruce-Pine-Fir Stud grade at 24" o.c.

FLOOR FRAMING

1. Floor finish: Per architectural plans.
2. Floor Sheathing: 3/4" tongue and groove wood product panels. Span index shall be 48/24 group 1 with exterior glue. Orientation shall be with face grain perpendicular to support. Provide 2x4 flat blocking at all unsupported edges where blocking is indicated. Sheathing adhesive shall be applied to framing members prior to placement of sheathing. Sheathing to be fully fastened prior to setting time of adhesive. Refer to detail (2/SD2.0).
3. Typical floor sheathing nails shall be:

   10d common nails at 6 inches o.c. along all boundaries and edges and 12 inches o.c. in field. Blocking is not required.

   Alternate: WSC 2L wood screws at 6" o.c. along boundaries and continuous edges and 12" o.c. in field. Screws shall be manufacture by Quick Drive USA, Inc. per ICBO # 5053.
4. Light metal plate connected wood floor trusses shall be designed to support the more critical of the loads and to limit deflections as follows. These requirements are in addition to those of ANSI/TPI 1.

Loading Conditions:	
Dead Load, including truss weight	16 PSF
Live Load	40 PSF
Partition Load	5 PSF
Lateral Load (at drag truss only)	As noted on plans or 2000# min.

Deflection Limitations, all cases:	
Vert. deflection under dead and live load	Span / 240 or 3/4" max.
Vert. deflection under live load only	Span / 480 or 3/4" max.
Vert. deflection between adj. trusses	1/2" max.

Prefabricated wood floor trusses to be provided by an approved fabricator with the shop drawings and calculations sealed by an Arizona registered engineer. Truss diagrams and keyed layouts shall be available to the field inspector at the job site at the time of floor nailing / framing inspection.

ROOF FRAMING

1. Roof Sheathing: 15/32" wood product panels. Span index shall be 32/16. Orientation shall be with face grain perpendicular to support. Refer to detail (2/SD2.0). Provide 2x4 flat blocking at all unsupported edges where blocking is indicated. Crickets and miscellaneous framing to overlay plywood sheathing. Nail sheathing as follows:
2. Typical roof sheathing nails shall be:

   8d common nails at 6 inches o.c. along all boundaries and edges and 12 inches o.c. in field. Blocking is not required.

   Alternate: 14 gauge x 2" long with 7/16" crown at 6" o.c. along all boundaries and edges and 12" o.c. in field. Blocking is not required.
   Alternate: 16 gauge x 2" long with 7/16" crown at 4" o.c. along all boundaries and edges and 8" o.c. in field. Blocking is not required.
3. Light metal plate connected wood roof trusses shall be designed to support the more critical of the loads and to limit deflections as follows: Trusses shall be cambered. Camber of the bottom chord of the trusses shall be as follows. These requirements are in addition to those of ANSI/TPI 1.

Loading Conditions:	
Dead Load, including truss weight	21 PSF
Live Load, typ.	16 PSF
Stucco Soffits	+10 PSF to bott. chord
Live Load, flat roofs (< 4 in 12 slope)	20 PSF
Lateral Load (at drag truss only)	As noted on plans or 2000# min.

Deflection Limitations, all cases:	
Vert. deflection under dead and live load	Span / 240 or 3/4" max.
Vert. deflection under live load	Span / 360 or 3/4" max.
Vert. deflection between adj. trusses	1/2" max.
Horz. deflection under dead and live load	3/8" max.

Camber:
Roof truss camber to equal dead load deflection

Prefabricated wood roof trusses to be provided by an approved fabricator with the shop drawings and calculations sealed by an Arizona registered engineer. Truss diagrams and keyed layouts shall be available to the field inspector at the job site at the time of roof nailing / framing inspection.

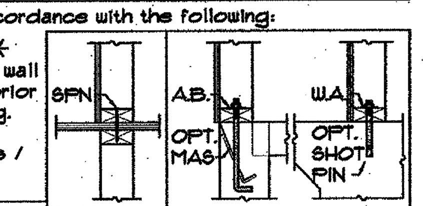
SHEAR WALL SCHEDULE: ARIZONA

BAI NO.: SHR-AZ

1. Sheath walls to resist racking forces in accordance with the following:

FRAMING LENGTH (n) * — Note: When an asterisk * accompanies the shear wall mark, apply sheathing prior to any adjacent framing.

A.B.'S @ N" OC / 16d @ N" OC — Spacing of anchor bolts / nails at sole plate.



MARK (n)	SHEATHING AND FASTENING	MINIMUM FRAMING: Top Plate	MINIMUM FRAMING: Panel Joints & blkg.	MINIMUM FRAMING: Btm. Plate	SOLE PL FASTENING TO WOOD (1,6,7)	MUD SILL FASTENING TO CONC. (2,8,9): Exterior	MUD SILL FASTENING TO CONC. (2,8,9): Interior
1	1/2" G.W.B. W/ 5d cooler nails at 7" o.c. along all edges parallel with framing and field (1,4,10)	2-2X	none	2X	16d Sinkers @ 12" O.C.	½" DIA. A.B. @ 72" O.C. OPT. MAS @ 32" O.C.	½" DIA. W.A. @ 72" O.C. OPT. SHOT PINS @ 18"
2	5/8" G.W.B. W/ 6d cooler nails at 7" o.c. along all edges and field (1,4,10)	2-2X	none	2X	16d Sinkers @ 8" O.C.	½" DIA. A.B. @ 72" O.C. OPT. MAS @ 32" O.C.	½" DIA. W.A. @ 72" O.C. OPT. SHOT PINS @ 18"
3	1/2" G.W.B. W/ 5d cooler nails at 4" o.c. along all edges parallel with framing and field (1,4,10)	2-2X	none	2X	16d Sinkers @ 8" O.C.	½" DIA. A.B. @ 64" O.C. OPT. MAS @ 32" O.C.	½" DIA. W.A. @ 64" O.C. OPT. SHOT PINS @ 12"
4	5/8" G.W.B. W/ 6d cooler nails at 4" o.c. along all edges and field (1,4,10)	2-2X	none	2X	16d Sinkers @ 8" O.C.	½" DIA. A.B. @ 64" O.C. OPT. MAS @ 32" O.C.	½" DIA. W.A. @ 64" O.C. OPT. SHOT PINS @ 12"
9	3/8" Wood product panel W/ 8d nails at 6" o.c. at panel edges 12" o.c. at field (1,3,5)	2-2X	2X	2X	16d Sinkers @ 4" O.C.	½" DIA. A.B. @ 32" O.C. OPT. MAS @ 32" O.C.	½" DIA. W.A. @ 32" O.C. OPT. SHOT PINS @ 6"
10	3/8" Wood product panel W/ 8d nails at 4" o.c. at panel edges 12" o.c. at field (1,3,5)	2-2X	2X	2X	16d Sinkers @ 3" O.C.	½" DIA. A.B. @ 24" O.C. OPT. MAS @ 24" O.C.	½" DIA. W.A. @ 24" O.C. OPT. SHOT PINS @ 4"
11	3/8" Wood product panel W/ 8d nails at 3" o.c. at panel edges 12" o.c. at field (1,3,5)	2-2X	2X	2X	16d Sinkers @ 2 1/2" O.C.	½" DIA. A.B. @ 16" O.C. OPT. MAS @ 16" O.C.	½" DIA. W.A. @ 16" O.C. NO SHOT PIN OPT.
12	3/8" Wood product panel W/ 8d nails at 2" o.c. at panel edges 12" o.c. at field (1,3,5)	2-2X	3X	2X	16d Sinkers @ 2" O.C.	½" DIA. A.B. @ 12" O.C. OPT. MAS @ 12" O.C.	½" DIA. W.A. @ 12" O.C. NO SHOT PIN OPT.

Footnote:
(1) Sheathing is included as part of required nail penetration.
(2) Shot pins to be 0.145" shank dia. X 2.875" shank length.
(3) Refer to specifications for plywood grading and permitted alternatives.
(4) At G.W.B. shear walls, sheet panels 4' wide and larger shall be applied parallel or perpendicular to studs. Sheet panels less than 4' wide shall be applied only perpendicular to studs.
(5) At wood product shearwalls, sheet panels shall be not less than 4' by 8' except at boundaries and changes where the minimum sheet panel dimension shall be 12 inches.
(6) Stagger is 1/2 inch.
(7) Space fasteners no closer than 1-1/2 inch. Over any 48 inch line of fasteners, the average spacing shall be at least as close as that given.
(8) 1/2" dia. wedge anchor (Simpson Wedge All, Ramset Redhead Trubolt, or Hilti Kwik Bolt) may be used in lieu of anchor bolts at interior shearwalls. Do not use wedge anchors at exterior shearwalls. Embed a minimum of 2 1/2".
(9) 1/2" dia. screw anchor (Simpson Titen HD or Ramset Redhead LDT) may be used in lieu of anchor bolts at exterior and interior shear walls. Embed a minimum of 2 1/2". Maintain minimum 1 3/4" edge distance.
(10) No. 6-1 1/4" type W or type S screws may be used in lieu of the 5d or 6d nails.

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REFER TO SN SHEETS FOR ALL NOTES AND SPECIFICATIONS

PROJECT:
Broadway at San Marcos

OWNER/DEVELOPER:
Liberty Homes

ARCHITECT/DESIGNER:
MDW Designs

SHEET:
PLAN 1786

ROOF FRAMING / UPPER SHEAR WALL PLAN

REFERENCE DATE: 03-28-07

ISSUED FOR:
CLIENT DELIVERY 03-29-07

ENGINEER OF RECORD:
Registered Professional Engineer (Civil) Certificate No. 35750 MICHAEL ERIC WILLIS Date Signed 3-29-07 ARIZONA, U.S.A.

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FILE NO: 12564
CAD NO. : 12564 1786 S2-3 FRM C
DRAWN BY : KER
ENGINEER : HI
SCALE : ¼" = 1'-0"

SHT NO. S2.3